Planning Proposal for a Mixed-Use Development

### 14-20 Parkes Street, Parramatta

#### TRAFFIC AND PARKING ASSESSMENT REPORT

27 July 2015

Ref 14872



Transport, Traffic and Parking Consultants 🏾 🦲 🥌

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### 1. INTRODUCTION

This report has been prepared to accompany a planning proposal to Parramatta City Council for a proposed mixed-use high rise mixed-use residential apartment building to be located at 14-20 Parkes Street, Parramatta (Figures 1 and 2).

The planning proposal will involve the demolition of the existing commercial buildings on the site to facilitate the construction of a new mixed-use residential apartment building with a ground floor retail component. Off-street car parking is to be accommodated in a multi-level above ground car park which will ultimately comply with Council's Parking Code requirements.

The purpose of this report is to assess the traffic and parking implications of the planning proposal and to that end this report:

- describes the site and provides details of the planning proposal
- reviews the road network in the vicinity of the site, and the traffic conditions on that road network
- estimates the traffic generation potential of the planning proposal, and assigns that traffic generation to the road network serving the site
- assesses the traffic implications of the planning proposal in terms of road network capacity
- reviews the geometric design features of the proposed car parking facilities for compliance with the relevant codes and standards
- assesses the adequacy and suitability of the quantum of off-street car parking provided on the site.



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### 2. PROPOSED DEVELOPMENT

#### Site

The subject site is located at the north-eastern corner of the Parkes Street and Wigram Street intersection. The site has a street frontage of approximately 60 metres in length to Parkes Street, and approximately 28 metres in length to Wigram Street. The site occupies an area of approximately 2,830m<sup>2</sup>.

The subject site is currently occupied by three separate commercial buildings, each with respective vehicular access driveways to Parkes Street. The commercial buildings comprise a mix of two and five-storey buildings, and have a cumulative floor area of approximately  $5,900m^2$ .

### **Proposed Development**

The planning proposal will involve the demolition of the existing buildings on the site to facilitate the construction of a new mixed-use residential apartment building.

The following schemes have been investigated as part of the planning proposal, with varying building heights and unit mix as follows:

Apartment	Scheme 1	Scheme 2	Scheme 3	Scheme 4	Scheme 5		
Туре	(15 Storeys)	(25 Storeys)	(32 Storeys)	(39 Storeys)	(44 Storeys)		
1 bedroom	31	52	73	97	112		
2 bedrooms	83	121	158	193	218		
3 bedrooms	9	16	24	31	36		
TOTAL	123	189	255	321	366		

Scheme 1 represents the existing development potential of the site under the current planning controls, whilst Scheme 5 represents the maximum development potential being considered as part of the planning proposal.

A retail component common to all schemes is proposed at Ground Level, with a cumulative floor area of  $600m^2$ .

Off-street car parking is proposed in an above ground multi-level car parking area, and the number of parking spaces to be provided will ultimately comply with Council's requirements. Vehicular access to the car parking facilities is to be provided via an entry/exit driveway located at the northern end of the Wigram Street site frontage.

Loading and servicing for planning proposed is expected to be undertaken by a variety of light commercial vehicles such as white vans, utilities and the like, which are capable of fitting into a conventional parking space. Garbage collection is expected to be undertaken by Council's waste contractor. These arrangements are to be reviewed in detail during the DA stage of the project.

### 3. TRAFFIC ASSESSMENT

### **Road Hierarchy**

The road hierarchy allocated to the road network in the vicinity of the site by the Roads and Maritime Services is illustrated on Figure 3.

The M4 Motorway is classified by the RMS as a *State Road* and provides the key east-west road link in the area, linking Concord to Emu Plains. It typically carries three traffic lanes in each direction in the vicinity of the site, with opposing traffic flows separated by a central median island. All intersections with the M4 Motorway are grade-separated.

James Ruse Drive is also classified by the RMS as a *State Road* and provides the key northsouth road link in the area, linking the Great Western Highway to Windsor Road. It typically carries three traffic lanes in each direction with turning bays provided at key locations.

Parkes Street and Hassall Street (east of Parkes Street) are classified by the RMS as a *Regional Road* which provides the key east-west *collector route* through the area. It typically carries two traffic lanes in each direction in the vicinity of the site, with kerbside parking permitted at selected locations only, outside commuter peak periods.

Wigram Street is a local, unclassified road which is primarily used to provide vehicular and pedestrian access to frontage properties. Kerbside parking is generally permitted along both sides of the road.

### **Existing Traffic Controls**

The existing traffic controls which apply to the road network in the vicinity of the site are illustrated on Figure 4. Key features of those traffic controls are:

- a 60 km/h SPEED LIMIT which applies to the Parkes Street and Harris Street (north of Parkes Street)
- a 50 km/h SPEED LIMIT which applies to all other local roads in the area





- TRAFFIC SIGNALS in Parkes Street where it intersects with Wigram Street and also Harris Street
- a NO RIGHT-TURN restriction westbound in Parkes Street into Wigram Street
- a NO RIGHT-TURN restriction eastbound in Parkes Street onto Harris Street.

#### **Sustainable Transport Options**

The proposed mixed-use building is located within the boundaries of the Parramatta City Centre where there is an extensive variety of sustainable transport options available such as train, bus, ferry, cycling and walking, as detailed below.

Parramatta Railway Station is located between Station Street and Argyle Street, approximately 400m west from the proposed development (approx. a 5 min walk). The Railway Station is a major railway interchange which services three train lines – The Blue Mountains Line, the Western Line and the Cumberland Line.

The Cumberland Line operates Monday to Friday only and offers two morning services and three afternoon services between Campbelltown and Blacktown. The Blue Mountains Line operates 7 days per week between Lithgow and Central, with generally one service per hour during off-peak periods, increasing to one service every 20-30min during peak periods. The Western Line operates 7 days per week between Emu Plains/Richmond and North Sydney/ North Shore, with generally one service approximately every 15min during off-peak periods, increasing to one service approximately every 5-10min during peak periods.

A major bus interchange is also located at Parramatta Railway Station which, as previously mentioned, is approximately 400m west from the proposed mixed-use development (approx. a 5 min walk).

In addition to the extensive range of train and bus services available in the Parramatta area, the Parramatta Rivercat Ferry service provides *express-only* services every hour between Circular Quay and Parramatta, 7 days a week. The Parramatta wharf is located at the northern

end of Charles Street, approximately 700m north of the proposed mixed-use building (approx. an 8 min walk).

#### **Existing Traffic Conditions**

An indication of the existing traffic conditions on the road network in the vicinity of the site is provided by peak period traffic surveys undertaken as part of this traffic study. The traffic surveys were undertaken in Wigram Street where it intersects with Parkes Street and also with Hassall Street. The results of the traffic surveys are reproduced in full in Appendix A and reveal that:

- two-way traffic flows in Parkes Street are typically in the order of 1,500 vph during peak periods
- two-way traffic flows in Wigram Street in front of the site are typically in the order of 350 vehicles per hour (vph) during peak periods
- two-way traffic flows in Hassall Street is typically less than 400 vph during peak periods.

#### **Projected Traffic Generation**

An indication of the traffic generation potential of the planning proposal is provided by reference to the Roads and Maritime Services publication *Guide to Traffic Generating Developments, Section 3 - Landuse Traffic Generation (October 2002).* 

The RMS *Guidelines* are based on extensive surveys of a wide range of land uses and nominates the following traffic generation rates which are applicable to the planning proposal:

#### High Density Residential Flat Buildings in Metropolitan Regional Centres

0.24 peak hour vehicle trips/dwelling

The RMS *Guidelines* also make the following observation in respect of high density residential flat buildings:

#### Definition

A *high density residential flat building* refers to a building containing 20 or more dwellings. This does not include aged or disabled persons housing. *High density residential flat buildings* are usually more than 5 levels, have basement level car parking and are located in close proximity to public transport services. The building may contain a component of commercial use.

#### Factors

The above rates include visitors, staff, service/delivery and on-street movements such as taxis and pick-up/set-down activities.

The RMS *Guidelines* do not nominate a traffic generation rate for small, local shops, referring only to major regional shopping centres incorporating supermarkets and department stores. For the purpose of this assessment therefore, the traffic generation rate of 2.0 peak hour vehicle trips per  $100m^2$  GFA nominated in the RMS *Guidelines* for *commercial premises* has been adopted in respect of the retail component of the planning proposal.

Application of the above traffic generation rates to the residential and retail components of each scheme is summarised in the table below, revealing that:

- under Scheme 1 which represents the existing development potential of the site under current planning controls yields a traffic generation potential of 42 vph
- under Scheme 5 which represents the maximum development potential of the site under the planning proposal yields a traffic generation potential of 100 vph
- the *nett* difference between Scheme 1 and Scheme 5 is 58 vph.

	Projec	ted Future Traine	Generation Poter	itiai	
Landuse	Scheme 1 (15 Storeys)	Scheme 2 (25 Storeys)	Scheme 3 (32 Storeys)	Scheme 4 (39 Storeys)	Scheme 5 (44 Storeys)
Residential	29.5 vph	45.4 vph	61.2 vph	77.0 vph	87.8 vph
Retail	12.0 vph				
TOTAL	41.5 vph	57.4 vph	73.2 vph	89.0 vph	99.8 vph

Projected Future Traffic Generation Potential

The projected future traffic generation potential of the site should however, be offset or *discounted* by the volume of traffic which could reasonably be expected to be generated by the existing commercial uses of the site, in order to determine the *nett increase (or decrease)* in traffic generation potential of the site expected to occur as a consequence of the planning proposal when compared with the previously approved development on the site.

Application of the *commercial premises* traffic generation rate nominated in the RMS *Guidelines* to the existing commercial buildings on the site which have a floor area of approximately  $5,900m^2$  yields a traffic generation potential of approximately 118 peak hour vehicle trips.

Accordingly, it is clear that *all* of the schemes being considered as part of this planning proposal would result in a *nett reduction* in the traffic generation potential of the site. In particular, it is noted that *Scheme 4* would result in a *nett reduction* in the traffic generation potential of the site of 18 vph as set out in the table below:

#### Projected Nett Change in Peak Hour Traffic Generation Potential

of the Site as a Consequence of the Planning Proposal	
Projected Future Traffic Generation Potential (Scheme 4):	99.8 vehicle trips
Less Existing Commercial Buildings Traffic Generation Potential (5900m <sup>2</sup> ):	-118.0 vehicle trips
NETT CHANGE IN TRAFFIC GENERATION POTENTIAL:	-18.2 vehicle trips

For the purposes of this assessment however, it has been assumed that the existing commercial buildings on the site are currently *vacant* and that *all* of the projected future traffic flows of up to 99.8 peak hour vehicle trips will be new or *additional* to the existing traffic flows currently using the adjacent road network.

#### **Traffic Implications - Road Network Capacity**

The traffic implications of planning proposals primarily concern the effects that any *additional* traffic flows may have on the operational performance of the nearby road network. Those effects can be assessed using the SIDRA program which is widely used by the RMS and many LGA's for this purpose. Criteria for evaluating the results of SIDRA analysis are reproduced in the following pages.

For the purposes of this assessment, the maximum traffic generation potential of the site under Scheme 4 has been adopted as it represents the maximum development potential of the site being considered as part of this planning proposal.

The detailed SIDRA *movement summaries* results pages are reproduced in full in Appendix B. The results of the SIDRA analysis are summarised in Table 3.1 and 3.2 below, revealing that:

### Wigram Street & Parkes Street Intersection

- the intersection currently operates at *Level of Service "B"* under the existing traffic demands with total average vehicle delays in the order of 23.5 seconds/vehicle
- under the projected future Scheme 4 traffic demands expected to be generated by the planning proposal, the intersection will continue to operate at *Level of Service "B"*, with increases in average vehicle delays of *less than* 3 seconds/vehicle.

### Wigram Street & Hassall Street Intersection

- the intersection currently operates at *Level of Service "A"* under the existing traffic demands with total average vehicle delays in the order of 4.6 seconds/vehicle
- under the projected future Scheme 4 traffic demands expected to be generated by the planning proposal, the intersection will continue to operate at *Level of Service "A"*, with increases in average vehicle delays of *less than* 1 second/vehicle.

In summary, it is clear that the planning proposal will not have any unacceptable traffic implications in terms of road network capacity.

TABLE 3.1 - RESULTS OF SIDKA ANALYSIS OF WIGRAM STREET & PARKES STREET INTERSECTION											
Key Indicators		Exis Traffic	sting Demand	Scheme 4 Projected Developmer Traffic Demand							
		AM	PM	AM	PM						
Level of Service		В	В	В	В						
Degree of Saturation		0.464	0.397	0.490	0.421						
Average Vehicle Delay (secs/ve	h)										
Wigram Street (south)	L T R	37.2 38.1 44.3	33.9 43.3 49.6	35.0 38.8 47.0	33.9 43.7 50.6						
Parkes Street (east)	L T	30.6 25.0	25.9 20.3	32.9 27.3	27.3 21.7						
Wigram Street (north)	L T <del>R</del>	38.7 39.7 44.3	42.6 43.8 48.4	36.6 44.3 48.9	41.0 45.0 49.5						
Parkes Street (west)	L T R	18.6 13.3 19.5	15.5 10.2 16.3	20.4 15.1 21.1	16.6 11.3 17.3						
TOTAL AVERAGE VEHICLI	E DELAY	23.5	21.1	25.6	22.3						
		WIG	PARX	WIG	PARP						

TABL WIGRAM	E 3.2 - RESULT STREET & HAS	<b>FS OF SIDRA</b> SSALL STRE	A ANALYSIS EET INTERSF	OF ECTION			
Key Indicators		Exis Traffic	sting Demand	Sche Projected I Traffic	eme 4 Development Demand		
		AM	PM	AM	РМ		
Level of Service		А	А	А	А		
Degree of Saturation		0.127	0.120	0.153	0.130		
Average Vehicle Delay (secs/vel	n)						
Wigram Street (south)	L R	7.8 7.5	7.6 7.5	7.8 7.5	7.6 7.5		
Hassall Street (east)	L T	4.6 0.0	4.6 0.0	4.6 0.0	4.6 0.0		
Hassall Street (west)	T R	0.2 4.8	0.1 4.7	0.3 4.9	0.2 4.8		
TOTAL AVERAGE VEHICLE	E DELAY	4.4	4.6	4.7	4.7		
		WIG	HASX	WIG HASP			

# **Criteria for Interpreting Results of Sidra Analysis**

### 1. Level of Service (LOS)

LOS	Traffic Signals and Roundabouts	Give Way and Stop Signs
'A'	Good operation.	Good operation.
'B'	Good with acceptable delays and spare capacity.	Acceptable delays and spare capacity.
'C'	Satisfactory.	Satisfactory but accident study required.
'D'	Operating near capacity.	Near capacity and accident study required.
Έ'	At capacity; at signals incidents will cause excessive	At capacity and requires other control mode.
	delays. Roundabouts require other control mode.	
'F'	Unsatisfactory and requires additional capacity.	Unsatisfactory and requires other control mode.

### 2. Average Vehicle Delay (AVD)

The AVD provides a measure of the operational performance of an intersection as indicated on the table below which relates AVD to LOS. The AVD's listed in the table should be taken as a guide only as longer delays could be tolerated in some locations (ie inner city conditions) and on some roads (ie minor side street intersecting with a major arterial route).

Level of Service	Average Delay per Vehicle (secs/veh)	Give Way and Stop Signs	
А	less than 14	Good operation.	Good operation.
В	15 to 28	Good with acceptable delays and spare capacity.	Acceptable delays and spare capacity.
С	29 to 42	Satisfactory.	Satisfactory but accident study required.
D	43 to 56	Operating near capacity.	Near capacity and accident study required.
E	57 to 70	At capacity; at signals incidents will cause excessive delays. Roundabouts require other control mode.	At capacity and requires other control mode.

### 3. Degree of Saturation (DS)

The DS is another measure of the operational performance of individual intersections.

For intersections controlled by traffic signals<sup>1</sup> both queue length and delay increase rapidly as DS approaches 1, and it is usual to attempt to keep DS to less than 0.9. Values of DS in the order of 0.7 generally represent satisfactory intersection operation. When DS exceeds 0.9 queues can be anticipated.

For intersections controlled by a roundabout or GIVE WAY or STOP signs, satisfactory intersection operation is indicated by a DS of 0.8 or less.

1

The values of DS for intersections under traffic signal control are only valid for cycle length of 120 secs.

### 4. PARKING IMPLICATIONS

### **Existing Kerbside Parking Restrictions**

The existing kerbside parking restrictions which apply to the road network in the vicinity of the site are illustrated on Figure 5 and comprise:

- NO STOPPING/NO PARKING restrictions along both sides of Parkes Street, including along the site frontage
- NO STOPPING restrictions along both sides of Hassall Street
- 4 HOUR PARKING restrictions along both sides of Wigram Street, and various sections along Hassall Street
- BUS ZONES at regular intervals along Parkes Street.

### **Off-Street Parking Provisions**

The *maximum* number of off-street parking spaces which may be provided on the site are specified in *Parramatta City Centre Local Environmental Plan 2007, Section 22C – Car Parking* document in the following terms:

Multi Dwelling Housing (1, 2 and 3 bedrooms)Residents:1 space per dwellingVisitors:1 space per 5 dwellings

Shops

1 space per 30m<sup>2</sup> of gross floor area

Application of the above parking requirements to each of the schemes outlined in the development proposal yields a *maximum* off-street parking provision between 168 and 459 parking spaces as set out below:



	On the	Site by Furfullati	u ony centre EEI	2007	
A nontmont Sizo	Scheme 1	Scheme 2	Scheme 3	Scheme 4	Scheme 5
Apartment Size	(15 Storeys)	(25 Storeys)	(32 Storeys)	(39 Storeys)	(44 Storeys)
Residents	123.0 spaces	189.0 spaces	255.0 spaces	321.0 spaces	366.0 spaces
Visitors	24.6 spaces	37.8 spaces	51.0 spaces	64.2 spaces	73.2 spaces
Retail	20.0 spaces	20.0 spaces	20.0 spaces	20.0 spaces	20.0 spaces
TOTAL	167.6 spaces	246.8 spaces	326 spaces	405.2 spaces	459.2 spaces

Maximum Parking Provisions Permitted On the Site by Parramatta City Centre LEP 2007

It is envisaged that the amount of car parking to be provided on the site will comply with Council's Parking Code requirements, noting that the Parramatta City Centre LEP 2007 specifies a *maximum* rather than a *minimum* parking requirement.

The geometric design layout of the proposed car parking facilities will be designed to comply with the relevant requirements specified in the Standards Australia publication *Parking Facilities Part 1 - Off-Street Car Parking AS2890.1* in respect of parking bay dimensions, ramp gradients and aisle widths.

In summary, the proposed parking facilities are capable of complying with the relevant requirements specified in both Council's Parking Code as well as the Australian Standards and it is therefore concluded that the proposed development could not be expected to have any unacceptable parking implications.

# APPENDIX A

### TRAFFIC SURVEY DATA

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0730 - 0745	17	17	20	11	5	22	92	0730 - 0745	0	0	0	0	0	0	0	0730 - 0745	17	17	20	11	5	22	92
0745 - 0800	14	18	23	19	2	12	88	0745 - 0800	0	0	0	0	0	1	1	0745 - 0800	14	18	23	19	2	13	89
0800 - 0815	11	42	30	13	5	1/	118	0800 - 0815	0	0	0	0	0	1	1	0800 - 0815	11	42	30	13	5	18	119
0815 - 0830	14	23	20	12	4	19	98	0815 - 0830	0	0	0	0	0	0	0	0815 - 0830	14	23	20	12	4	19	98
0845 - 0900	19	41	33	14	6	20	141	0845 - 0900	0	0	0	0	0	0	0	0845 - 0900	19	41	33	14	6	20 17	141
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0730 - 0830	56	100	99	55	16	70	396	0730 - 0830	0	0	0	0	0	2	2	0730 - 0830	56	100	99	55	16	72	398
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Time Per	T	R	Ľ	R	L	T	тот	Time Per	I	R	Ŀ	R	L	T	тот	Time Per	Τ	R	L	R	L	I	тот
1600 - 1615	20	52	17	16	2	9	116	1600 - 1615	0	0	0	0	0	0	0	1600 - 1615	20	52	17	16	2	9	116
1615 - 1630	12	35	12	15	3	10	87	1615 - 1630	0	0	0	0	0	0	0	1615 - 1630	12	35	12	15	3	10	87
1630 - 1645	14	57	11	29	7	6	124	1630 - 1645	0	0	0	0	0	0	0	1630 - 1645	14	57	11	29	7	6	124
1645 - 1700	18	38	17	22	3	8	106	1645 - 1700	0	0	0	0	0	0	0	1645 - 1700	18	38	17	22	3	8	106
1700 - 1715	18	63	11	14	6	9	121	1700 - 1715	0	0	0	0	0	0	0	1700 - 1715	18	63	11	14	6	9	121
1715 - 1730	14	40	13	23	3	12	105	1715 - 1730	0	0	0	0	0	0	0	1715 - 1730	14	40	13	23	3	12	105
1730 - 1745	20	36	7	18	7	14	102	1730 - 1745	0	0	0	0	0	0	0	1730 - 1745	20	36	7	18	7	14	102
1745 - 1800	9	30	19	14	8	10	90	1745 - 1800	0	0	0	0	0	0	0	1745 - 1800	9	30	19	14	8	10	90
Per End	125	351	107	151	39	78	851	Per End	0	0	0	0	0	0	0	Per End	125	351	107	151	39	78	851
Lights	WF	ST	SO	UTH	F۵	ST		Heavies	WF	ST	SO	ЛТН	E4	ST		Combined	WF	ST	SO	UTH	F۵	ST	
Ligito	Hass	all St	Wiar	am St	Hass	all St		11001100	Hass	all St	Wigr	am St	Hass	all St		<u>o o mono u</u>	Hass	all St	Wigr	am St	Hass	all St	
Peak Per	77033 T	R	L	R	L	<i>ції</i> О(	тот	Peak Per	T	R	L	R	L	<u>ал ос</u> Т	тот	Peak Per	T	R	L	R	L	T	тот
1600 - 1700	64	182	57	82	15	33	433	1600 - 1700	0	0	0	0	0	0	0	1600 - 1700	64	182	57	82	15	33	433
1615 - 1715	62	193	51	80	19	33	438	1615 - 1715	0	0	0	0	0	0	0	1615 - 1715	62	193	51	80	19	33	438
1630 - 1730	64	198	52	88	19	35	456	1630 - 1730	0	0	0	0	0	0	0	1630 - 1730	64	198	52	88	19	35	456
1645 - 1745	70	177	48	77	19	43	434	1645 - 1745	0	0	0	0	0	0	0	1645 - 1745	70	177	48	77	19	43	434
1700 - 1800	61	169	50	69	24	45	418	1700 - 1800	0	0	0	0	0	0	0	1700 - 1800	61	169	50	69	24	45	418
PEAK HR	64	198	52	88	19	35	456	PEAK HR	0	0	0	0	0	0	0	PEAK HR	64	198	52	88	19	35	456
Peds	WE	ST	SO	UTH	EA	ST	1				Hass	all St								Hass	all St		
Time Per	Hass	all St	Wigr	am St	Hass	all St	тот			C	262	262							0	152	152		•
1600 - 1615	2	4	1	11	9	9	44																
1615 - 1630	3	4		6	4	4	44	Hours 1			0	64	64		•	-		•	35	35	0		
1630 - 1645	4	2		9	1	9	70																
1645 - 1700	2	27	1	11	9	9	47	Hours 2															
1700 - 1715	2	27	1	18	ł	8	53				0	198	198						19	19	0		
1715 - 1730	3	51	1	13	1	0	54	Hours 3							•			*					
1730 - 1745	6	9	1	13	1	8	90			◀	- 87	87	0			-			•	54	54	0	
1745 - 1800	3	5		6	1	1	52	Hours 4							`								
Per End	2	89	8	37	7	8	454								52	88							
								Hours 5							52	88							
Deals D	WE	51	SO		ÉA	51	TOT	_		<u>PM</u>					0	0				N/			
Peak Per	Hass	all St	Wigr	am St	Hass	all St	101			1030	- 1730					_	0			N			
1600 - 1700	12	27		57	4		205										217						
1615 - 1715	1:	3U 27		14	4	iu Ic	214								140		217		-		>		
1630 - 1/30	12	21 54		5	4	HO 16	224	_							0		+			4			
1700 - 1800	1	24 22		50	3	17	244	_							v	Wiaram St				· ·			
1700 - 1800		2			3		243									ngian 3t							
PEAK HR	1:	27	5	51	4	6	224													C	Copyrig	ht ROA	R DATA

	<b>R.O</b>	. <b>A.</b> R	DATA									С	lient	: Varga Traff	ic Planning				
	Reliat	ble, Ori	iqinal & Autl	hentic	Result	s						Job N	o/Name	: 5450 PARF	RAMATTA W	/igram \$	St Cour	nts	
DA	Ph.88	196847	, Fax 88196	849, M	lob.041	8-23901	Э					Day	/Date	: Wednesday	v 10th Dece	mber 2	014		
												ĺ							
									т	OTAL VO	LUMES								
				P	M					FOR C	JUNT								
										PER	OD			N					
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					0	476	476	; —				0	276	276					
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				ŀ	lassal	l St								Has	sall St		-		
				-	-	185	185	0			-		117	117	0				
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	R.O	.A.F	R. D	ATA	4									Client		: Var	ga Tra	affic P	lannin	g							
	Relia	ble, C	)rigina	al & A	uthen	ntic Re	esults							Job No/Na	ame	: 545	0 PAF	RRAM	ATTA	Wigra	am St	Count	S				
DN	Ph.88	196847	7, Fax	881968	349, M	ob. 041	18 239	019					1	Day/Da	te	:We	dnesc	ay 10	th Deo	cembe	er 201	4					-
Lights	1		1		WEST			SOUTH	<u> </u>		EAST			<u>Heavies</u>		NORTI	H A		WEST			SOUTI	H		EAST		
Time Bor		igram	St	P	arkes 3	St D		igram	St	, ,	arkes a	St D	TOT	Time Bor		vigram	St	, P	arkes :			igram T	St	, ,	arkes S	ot D	TOT
0700_0715	<u> </u>	<u> </u>	2	2	201	<u> </u>	1	<u>1</u>	<u> </u>	<u> </u>	<u> </u>	<u> </u>	260	0700_0715		<u> </u>	<u> </u>		<u> </u>	1		<u> </u>	<u> </u>	1	<u> </u>	<u> </u>	101
0715 - 0730	7	4	4	11	189	13	4	18	4	8	112	0	374	0715 - 0730	0	0	0	0	5	0	0	0	0	0	8	0	13
0730 - 0745	6	9	3	15	179	17	3	16	2	4	140	0	394	0730 - 0745	0	0	0	0	5	0	1	0	0	0	4	0	10
0745 - 0800	11	16	4	13	195	29	6	47	10	10	150	0	491	0745 - 0800	0	0	0	0	7	0	0	0	0	0	10	0	17
0800 - 0815	16	14	6	9	181	21	3	31	10	7	176	0	474	0800 - 0815	0	0	1	0	7	1	1	0	0	1	7	0	18
0815 - 0830	14	26	12	12	148	22	9	56	17	10	176	0	502	0815 - 0830	0	0	0	0	5	0	0	0	0	0	10	0	15
0830 - 0845	24	15	4	8	154	32	7	38	3	7	194	0	486	0830 - 0845	0	0	0	0	7	1	1	0	0	1	7	0	17
0845 - 0900	26	22	10	13	136	26	5	34	7	11	188	0	478	0845 - 0900	0	0	0	0	3	1	0	0	0	1	11	0	16
Period End	108	111	45	84	1383	176	38	254	59	63	1247	0	3568	Period End	0	0	1	0	44	4	3	0	0	4	63	0	119
Lights	Ν	IORTI	4		WEST			SOUTH	4		EAST			<u>Heavies</u>		NORTI	Н		WEST	•		SOUTI	Н		EAST		
	W	igram	St	Р	arkes S	St	N	igram	St	F	Parkes S	St			V	Vigram	St	F	Parkes	St	N	ligram	St	F	Parkes S	St	
Peak Time	Ŀ	<u>T</u>	<u>R</u>	Ŀ	Ţ	<u>R</u>	Ŀ	Ţ	<u>R</u>	Ŀ	Ţ	<u>R</u>	тот	Peak Time	Ŀ	T	<u>R</u>	L	<u>T</u>	<u>R</u>	Ŀ	Ţ	<u>R</u>	L	Ţ	<u>R</u>	тот
0700 - 0800	28	34	13	42	764	75	14	95	22	28	513	0	1628	0700 - 0800	0	0	0	0	22	1	1	0	0	1	28	0	53
0715 - 0815	40	43	17	48	744	80	16	112	26	29	578	0	1733	0715 - 0815	0	0	1	0	24	1	2	0	0	1	29	0	58
0730 - 0830	47	65	25	49	703	89	21	150	39	31	642	0	1861	0730 - 0830	0	0	1	0	24	1	2	0	0	1	31	0	60
0745 - 0845	65	/1	26	42	678	104	25	1/2	40	34	696	0	1953	0745 - 0845	0	0	1	0	26	2	2	0	0	2	34	0	67
0800 - 0900	80	11	32	42	619	101	24	159	37	30	734	0	1940	0800 - 0900	0	0	ļ	0	22	3	2	0	0	3	30	0	00
PEAK HOUR	65	71	26	42	678	104	25	172	40	34	696	0	1953	PEAK HOUR	0	0	1	0	26	2	2	0	0	2	34	0	67
Combined	N	IORT	4		WEST		5	SOUTH	4		EAST		1	Peds		NORTI	Η		WEST	•		SOUTI	H		EAST		
	W	igram	St	Р	arkes S	St	и	igram ·	St	F	Parkes S	St			V	Vigram	St	F	Parkes	St	и	'igram	St	F	Parkes S	St	
Time Per	L	I	<u>R</u>	L	I	<u>R</u>	L	I	<u>R</u>	L	I	<u>R</u>	тот	Time Per	UNC	CLASSI	FIED	UNC	LASSI	<u>FIED</u>	UNC	LASSI	FIED	UNC	LASSI	FIED	тот
0700 - 0715	4	5	2	3	206	17	1	14	6	7	117	0	382	0700 - 0715		1			1			2			7		11
0715 - 0730	7	4	4	11	194	13	4	18	4	8	120	0	387	0715 - 0730		1			1			4			5		11
0730 - 0745	6	9	3	15	184	17	4	16	2	4	144	0	404	0730 - 0745		2			8			4			7		21
0745 - 0800	11	16	4	13	202	29	6	47	10	10	160	0	508	0745 - 0800		3			13			0			10		26
0800 - 0815	16	14	7	9	188	22	4	31	10	8	183	0	492	0800 - 0815		1			1			1			12		15
0815 - 0830	14	26	12	12	153	22	9	56	17	10	186	0	517	0815 - 0830		0			10			2			12		35
0830 - 0845	24	22	4	0	101	33 27	5	30	3	0 12	100	0	503 494	0830 - 0845		4			14			6			12		38
Period End	108	111	46	84	1427	180	41	254	59	67	1310	0 0	3687	Period End		28			16			19			72		198
Combined			1		WEST			OUT			EAST			Pada					WEST						EVET		
<u>combined</u>	1	loram	1 St	P	orkos (	S#	IA.	/iaram	ר קי		EAJ I Parkos (	S#		Peas	V	Viaram	п Sf		VVE31	54	14	liaram	п \$f		EAJ I Parkos 9	*	
Peak Time	L	T	R	L	T	R	L.	Т	R	Ľ	Т	R	тот	Peak Per	UNC	CLASSI	FIED	UNC	LASSI	FIED	UNC		FIED	UNC	LASSI	FIED	тот
0700 - 0800	28	34	13	42	786	76	15	95	22	29	<u>.</u> 541	0	1681	0700 - 0800		7			23			10			29		69
0715 - 0815	40	43	18	48	768	81	18	112	26	30	607	0	1791	0715 - 0815		7			23			9			34		73
0730 - 0830	47	65	26	49	727	90	23	150	39	32	673	0	1921	0730 - 0830		14			40			5			38		97
0745 - 0845	65	71	27	42	704	106	27	172	40	36	730	0	2020	0745 - 0845		16			55			3			43		117
0800 - 0900	80	77	33	42	641	104	26	159	37	38	769	0	2006	0800 - 0900		21			56			9			43		129
PEAK HOUR	65	71	27	42	704	106	27	172	40	36	730	0	2020	PEAK HR		16	l		55			3			43		117

<b>A</b>	R.C	).A.	r da'	ТА												Cli	ent	: Var	ga Traffi	c Plan	ning					
	Relia	ble, O	riginal 8	Auther	ntic R	esults	5								Jol	b No/I	Name	: 545	0 PARR	АМАТ	TA Wigra	am St Cou	nts			
DN	Ph.88	319684	47, Fax 8	3819684	9, Mot	o. 041	8 239	019								Dav/	/Date	:We	dnesdav	10th I	Decembe	er 2014				
																,										
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		Hours	s 1	Hours 2		Hou	irs 3		Hour	rs 4		Hours	s 5				A.F	-								
					Γ -																					
					W	igram	n St														Wigra	am St				
					•																					
	<u> </u>	I PEA	ĸ				1				_												_	тот		LUMES
	074	15 - 08	845		214		162																	FO	R PER	RIOD
				1	214	0	163	0													338	1		C	<u>оинт</u>	ED
				26	0	71		65		_											338	264				
				27		71	•	65			_										0	265	_			
																							_			
		Parke	es St	┛┥		•																•				
	28	824	852	►					≜	2	6 7	783	809													
	0	42	42							0	0	0					48		1643	16	591 <b></b>	44	1550	)	1594	. —>
				<u> </u>		$\left( \widetilde{\mathbf{q}} \right)^{*}$	)				_					=		Park	es St							
	26	678	704	•	Ŕ	DA	`		-	730	696	6 34	4			4							Park	res St		
									_							-	1397		1330	67		<b>■</b> 137	7	1310		67
	2	104	106	<b>_</b>					$\mathbf{I}$	36	34	2														
	784	747	37	┛		•		┍►			766	6 73	30	36	_						<b>_</b>					
										Pa	n kes	s St														
						1					_										354	8				
				27		172		40		_	_										351	350				
				25	T	172	4	40		_	_										3	358				
				2	239	0	209	0			_	_	©	Copyrig	ght ROAR D	DATA										
					237		213			_	_											▼				
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						/ <b>-</b>					_										1477					
					W	ıgram	i St				_										wigra	am St				

	R.O.A.R. DATA													Client		: Var	ga Tra	affic P	lannin	ng							
	Relia	ble, C	Drigina	al & A	luther	ntic Re	esults							Job No/Na	ame	: 545	0 PAF	RRAM	ATTA	Wigra	am St	Count	ts				
DA	Ph.88	196847	7, Fax	88196	849, M	ob. 04′	18 239	019						Day/Da	te	:We	dnesc	lay 10	th Deo	cembe	er 201	4					
Lights	1	NORTH	4		WEST	•	<b>u</b> ,	SOUTH	H		EAST			<u>Heavies</u>		NORTI	Н		WEST			SOUT	Н		EAST		
	И	igram/	St	F	Parkes	St	W	igram	St	F	Parkes S	St			И	Vigram	St	F	Parkes S	St	И	/igram	St	F	Parkes S	St	
Time Per	L	Ţ	<u>R</u>	L	Ţ	<u>R</u>	L	Ţ	<u>R</u>	L	<u>T</u>	<u>R</u>	тот	Time Per	L	T	<u>R</u>	L	<u>T</u>	<u>R</u>	L	T	<u>R</u>	L	I	<u>R</u>	тот
1600 - 1615	15	34	12	15	137	24	13	19	16	17	166	0	468	1600 - 1615	0	0	0	0	3	0	1	0	1	0	6	0	11
1615 - 1630	6	26	7	13	153	24	10	11	11	21	176	0	458	1615 - 1630	0	0	0	0	5	1	0	0	0	0	4	0	10
1630 - 1645	18	28	6	15	164	26	16	14	10	10	160	0	467	1630 - 1645	0	0	0	0	4	0	1	0	0	0	2	0	7
1645 - 1700	12	21	10	15	158	29	9	21	7	10	162	0	454	1645 - 1700	0	0	0	0	4	1	0	0	0	0	2	0	7
1700 - 1715	16	35	14	7	152	27	13	26	6	6	170	0	472	1700 - 1715	0	0	0	0	1	0	0	0	0	0	1	0	2
1715 - 1730	8	31	10	16	143	26	13	20	12	14	167	0	460	1715 - 1730	0	0	0	0	4	1	1	0	0	0	2	0	8
1730 - 1745	10	22	7	9	159	32	14	21	8	13	169	0	464	1730 - 1745	0	0	0	0	1	0	0	0	0	0	1	0	2
1745 - 1800	12	9	5	10	162	37	18	21	10	11	163	0	458	1745 - 1800	0	0	0	0	2	1	1	0	0	0	1	0	5
Period End	97	206	71	100	1228	225	106	153	80	102	1333	0	3701	Period End	0	0	0	0	24	4	4	0	1	0	19	0	52
Lights	1	ORTH	4		WEST		9	SOUTI	Η		EAST	-		Heavies	I	NORTI	H		WEST		9	SOUT	Н		EAST		1
	И	/igram	St	F	Parkes S	St	W	ligram	St	F	Parkes S	St			И	Vigram	St	F	Parkes S	St	И	/igram	St	F	Parkes S	St	
Peak Time	L	Ι	R	L	Ţ	R	L	Ţ	R	L	T	R	тот	Peak Time	L	T	<u>R</u>	L	T	<u>R</u>	L	T	<u>R</u>	L	Ι	R	тот
1600 - 1700	51	109	35	58	612	103	48	65	44	58	664	0	1847	1600 - 1700	0	0	0	0	16	2	2	0	1	0	14	0	35
1615 - 1715	52	110	37	50	627	106	48	72	34	47	668	0	1851	1615 - 1715	0	0	0	0	14	2	1	0	0	0	9	0	26
1630 - 1730	54	115	40	53	617	108	51	81	35	40	659	0	1853	1630 - 1730	0	0	0	0	13	2	2	0	0	0	7	0	24
1645 - 1745	46	109	41	47	612	114	49	88	33	43	668	0	1850	1645 - 1745	0	0	0	0	10	2	1	0	0	0	6	0	19
1700 - 1800	46	97	36	42	616	122	58	88	36	44	669	0	1854	1700 - 1800	0	0	0	0	8	2	2	0	0	0	5	0	17
PEAK HOUR	51	109	35	58	612	103	48	65	44	58	664	0	1847	PEAK HOUR	0	0	0	0	16	2	2	0	1	0	14	0	35
Combined			4		WEST		9	SOUTI			FAST		1	Peds		NORTI	H		WEST			SOUT	H		FAST		1
oomsinou	и	liaram	St	F	Parkes	St	W	iaram	St	F	Parkes S	St		1000	и	Viaram	St	P	Parkes	St	и	/iaram	St	F	Parkes S	St	
Time Per	L	Т	R	L	Т	R	L	T	R	L	Т	R	тот	Time Per	UNC	CLASSI	FIED	UNC	LASSI	FIED	UNC	LASSI	FIED	UNC	LASSI	FIED	тот
1600 - 1615	15	34	12	15	140	24	14	19	17	17	172	0	479	1600 - 1615		5			8			1			4		18
1615 - 1630	6	26	7	13	158	25	10	11	11	21	180	0	468	1615 - 1630		3			18			3			9		33
1630 - 1645	18	28	6	15	168	26	17	14	10	10	162	0	474	1630 - 1645		3			10			2			8		23
1645 - 1700	12	21	10	15	162	30	9	21	7	10	164	0	461	1645 - 1700		5			11			6			6		28
1700 - 1715	16	35	14	7	153	27	13	26	6	6	171	0	474	1700 - 1715		6			9			1			11		27
1715 - 1730	8	31	10	16	147	27	14	20	12	14	169	0	468	1715 - 1730		4			6			1			5		16
1730 - 1745	10	22	7	9	160	32	14	21	8	13	170	0	466	1730 - 1745		3			4			4			3		14
1745 - 1800	12	9	5	10	164	38	19	21	10	11	164	0	463	1745 - 1800		1			3			2			0		6
Period End	97	206	71	100	1252	229	110	153	81	102	1352	0	3753	Period End		30			69			20			46		165
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	И	igram/	St	F	Parkes	St	W	igram	St	ŀ	Parkes S	St			И	Vigram	St	P	Parkes S	St	И	/igram	St	F	Parkes S	St	
Peak Time	L	<u>T</u>	<u>R</u>	L	<u>T</u>	<u>R</u>	L	T	<u>R</u>	L	<u>T</u>	<u>R</u>	тот	Peak Per	UNC	CLASSI	FIED	UNC	LASSI	<u>FIED</u>	UNC	LASSI	<u>FIED</u>	UNC	LASSI	FIED	тот
1600 - 1700	51	109	35	58	628	105	50	65	45	58	678	0	1882	1600 - 1700		16			47			12			27		102
1615 - 1715	52	110	37	50	641	108	49	72	34	47	677	0	1877	1615 - 1715		17			48			12			34		111
1630 - 1730	54	115	40	53	630	110	53	81	35	40	666	0	1877	1630 - 1730		18			36			10			30		94
1645 - 1745	46	109	41	47	622	116	50	88	33	43	674	0	1869	1645 - 1745		18			30			12			25		85
1700 - 1800	46	97	36	42	624	124	60	88	36	44	674	0	1871	1700 - 1800		14			22			8	. <u> </u>		19		63
		100	25	58	628	105	50	65	45	58	678	0	1882	PEAK HR		16			47			12			27		102

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### **APPENDIX B**

### SIDRA RESULTS

#### 🥮 Site: Existing AM

Wigram Street & Hassall Street Intersection Stop (Two-Way)

Moven	nent Pe	rformance - V	ehicles								
Mov ID	OD Mov	Demand Total veh/h	Flows HV %	Deg. Satn v/c	Average Delay sec	Level of Service	95% Back o Vehicles veh	of Queue Distance m	Prop. Queued	Effective Stop Rate per veh	Average Speed km/h
South: \	Wigram (	Street (south)									
1	L2	120	0.0	0.127	7.8	LOS A	0.5	3.2	0.14	0.92	45.0
3	R2	56	0.0	0.127	7.5	LOS A	0.5	3.2	0.14	0.92	44.6
Approa	ch	176	0.0	0.127	7.7	LOS A	0.5	3.2	0.14	0.92	44.9
East: H	assall St	reet (east)									
4	L2	21	0.0	0.052	4.6	LOS A	0.0	0.0	0.00	0.11	48.9
5	T1	79	1.3	0.052	0.0	LOS A	0.0	0.0	0.00	0.11	49.3
Approa	ch	100	1.0	0.052	1.0	NA	0.0	0.0	0.00	0.11	49.2
West: H	lassall S	treet (west)									
11	T1	66	0.0	0.104	0.2	LOS A	0.5	3.7	0.16	0.37	47.5
12	R2	154	0.0	0.104	4.8	LOS A	0.5	3.7	0.16	0.37	46.6
Approa	ch	220	0.0	0.104	3.4	NA	0.5	3.7	0.16	0.37	46.9
All Vehi	cles	496	0.2	0.127	4.4	NA	0.5	3.7	0.12	0.52	46.6

Level of Service (LOS) Method: Delay (RTA NSW).

Vehicle movement LOS values are based on average delay per movement

Minor Road Approach LOS values are based on average delay for all vehicle movements.

NA: Intersection LOS and Major Road Approach LOS values are Not Applicable for two-way sign control since the average delay is not a good LOS measure due to zero delays associated with major road movements.

SIDRA Standard Delay Model is used. Control Delay includes Geometric Delay.

Gap-Acceptance Capacity: SIDRA Standard (Akçelik M3D).

HV (%) values are calculated for All Movement Classes of All Heavy Vehicle Model Designation.

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#### 😳 Site: Existing PM

Wigram Street & Hassall Street Intersection Stop (Two-Way)

Movem	ient Pe	erformance - Ve	hicles								
Mov ID	OD Mov	Demand F Total veh/h	Flows HV %	Deg. Satn v/c	Average Delay sec	Level of Service	95% Back o Vehicles veh	of Queue Distance m	Prop. Queued	Effective Stop Rate per veh	Average Speed km/h
South: V	Vigram	Street (south)									
1	L2	52	0.0	0.097	7.6	LOS A	0.3	2.2	0.08	0.95	45.0
3	R2	88	0.0	0.097	7.5	LOS A	0.3	2.2	0.08	0.95	44.6
Approac	:h	140	0.0	0.097	7.5	LOS A	0.3	2.2	0.08	0.95	44.8
East: Ha	assall S	treet (east)									
4	L2	19	0.0	0.028	4.6	LOS A	0.0	0.0	0.00	0.19	48.5
5	T1	35	0.0	0.028	0.0	LOS A	0.0	0.0	0.00	0.19	48.9
Approac	:h	54	0.0	0.028	1.6	NA	0.0	0.0	0.00	0.19	48.8
West: H	assall S	Street (west)									
11	T1	64	0.0	0.120	0.1	LOS A	0.6	4.5	0.11	0.40	47.5
12	R2	198	0.0	0.120	4.7	LOS A	0.6	4.5	0.11	0.40	46.6
Approac	:h	262	0.0	0.120	3.6	NA	0.6	4.5	0.11	0.40	46.8
All Vehic	cles	456	0.0	0.120	4.6	NA	0.6	4.5	0.09	0.55	46.4

Level of Service (LOS) Method: Delay (RTA NSW).

Vehicle movement LOS values are based on average delay per movement

Minor Road Approach LOS values are based on average delay for all vehicle movements.

NA: Intersection LOS and Major Road Approach LOS values are Not Applicable for two-way sign control since the average delay is not a good LOS measure due to zero delays associated with major road movements.

SIDRA Standard Delay Model is used. Control Delay includes Geometric Delay.

Gap-Acceptance Capacity: SIDRA Standard (Akcelik M3D).

HV (%) values are calculated for All Movement Classes of All Heavy Vehicle Model Designation.

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# 😳 Site: Proposed AM

Wigram Street & Hassall Street Intersection Stop (Two-Way)

Movem	ent Perfo	ormance - Ve	hicles								
Mov ID	OD Mov	Demand F Total veh/h	Flows HV %	Deg. Satn v/c	Average Delay sec	Level of Service	95% Back c Vehicles veh	of Queue Distance m	Prop. Queued	Effective Stop Rate per veh	Average Speed km/h
South: V	Vigram Stre	eet (south)									
1	L2	138	0.0	0.153	7.8	LOS A	0.6	3.9	0.14	0.93	45.0
3	R2	74	0.0	0.153	7.5	LOS A	0.6	3.9	0.14	0.93	44.6
Approac	h	212	0.0	0.153	7.7	LOS A	0.6	3.9	0.14	0.93	44.9
East: Ha	ssall Stree	et (east)									
4	L2	35	0.0	0.060	4.6	LOS A	0.0	0.0	0.00	0.17	48.6
5	T1	79	1.3	0.060	0.0	LOS A	0.0	0.0	0.00	0.17	49.0
Approac	h	114	0.9	0.060	1.4	NA	0.0	0.0	0.00	0.17	48.9
West: Ha	assall Stree	et (west)									
11	T1	66	0.0	0.108	0.3	LOS A	0.6	3.9	0.17	0.38	47.5
12	R2	160	0.0	0.108	4.9	LOS A	0.6	3.9	0.17	0.38	46.6
Approac	h	226	0.0	0.108	3.5	NA	0.6	3.9	0.17	0.38	46.8
All Vehic	les	552	0.2	0.153	4.7	NA	0.6	3.9	0.13	0.54	46.5

Level of Service (LOS) Method: Delay (RTA NSW).

Vehicle movement LOS values are based on average delay per movement

Minor Road Approach LOS values are based on average delay for all vehicle movements.

NA: Intersection LOS and Major Road Approach LOS values are Not Applicable for two-way sign control since the average delay is not a good LOS measure due to zero delays associated with major road movements.

SIDRA Standard Delay Model is used. Control Delay includes Geometric Delay.

Gap-Acceptance Capacity: SIDRA Standard (Akçelik M3D).

HV (%) values are calculated for All Movement Classes of All Heavy Vehicle Model Designation.

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# 😳 Site: Proposed PM

Wigram Street & Hassall Street Intersection Stop (Two-Way)

Movem	ent Perf	ormance - Ve	hicles								
Mov ID	OD Mov	Demand F Total veh/h	lows HV %	Deg. Satn v/c	Average Delay sec	Level of Service	95% Back c Vehicles veh	of Queue Distance m	Prop. Queued	Effective Stop Rate per veh	Average Speed km/h
South: V	Vigram St	reet (south)									
1	L2	58	0.0	0.107	7.6	LOS A	0.3	2.4	0.08	0.96	45.0
3	R2	95	0.0	0.107	7.5	LOS A	0.3	2.4	0.08	0.96	44.6
Approac	h	153	0.0	0.107	7.5	LOS A	0.3	2.4	0.08	0.96	44.7
East: Ha	issall Stre	et (east)									
4	L2	56	0.0	0.048	4.6	LOS A	0.0	0.0	0.00	0.33	47.7
5	T1	35	0.0	0.048	0.0	LOS A	0.0	0.0	0.00	0.33	48.1
Approac	h	91	0.0	0.048	2.8	NA	0.0	0.0	0.00	0.33	47.9
West: Ha	assall Stre	eet (west)									
11	T1	64	0.0	0.130	0.2	LOS A	0.7	4.9	0.16	0.41	47.3
12	R2	216	0.0	0.130	4.8	LOS A	0.7	4.9	0.16	0.41	46.5
Approac	h	280	0.0	0.130	3.8	NA	0.7	4.9	0.16	0.41	46.7
All Vehic	les	524	0.0	0.130	4.7	NA	0.7	4.9	0.11	0.55	46.3

Level of Service (LOS) Method: Delay (RTA NSW).

Vehicle movement LOS values are based on average delay per movement

Minor Road Approach LOS values are based on average delay for all vehicle movements.

NA: Intersection LOS and Major Road Approach LOS values are Not Applicable for two-way sign control since the average delay is not a good LOS measure due to zero delays associated with major road movements.

SIDRA Standard Delay Model is used. Control Delay includes Geometric Delay.

Gap-Acceptance Capacity: SIDRA Standard (Akçelik M3D).

HV (%) values are calculated for All Movement Classes of All Heavy Vehicle Model Designation.

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#### Site: Existing AM

Wigram Street & Parkes Street Intersection

Signals - Fixed Time Isolated Cycle Time = 120 seconds (User-Given Cycle Time)

Mover	nent Per	formance - V	ehicles								
Mov ID	OD Mov	Demand Total veh/h	Flows HV %	Deg. Satn v/c	Average Delay sec	Level of Service	95% Back Vehicles veh	of Queue Distance m	Prop. Queued	Effective Stop Rate per veh	Average Speed km/h
South:	Wigram S	Street (south)									
1	L2	27	7.4	0.122	37.2	LOS C	2.8	20.3	0.76	0.65	35.3
2	T1	172	0.0	0.456	38.1	LOS C	8.4	58.5	0.85	0.72	32.5
3	R2	40	0.0	0.456	44.3	LOS D	8.4	58.5	0.87	0.74	33.4
Approa	ich	239	0.8	0.456	39.0	LOS C	8.4	58.5	0.84	0.71	33.0
East: P	arkes Stre	eet (east)									
4	L2	36	5.6	0.459	30.6	LOS C	15.5	112.9	0.75	0.67	39.2
5	T1	730	4.7	0.459	25.0	LOS B	15.6	113.4	0.75	0.66	42.4
Approa	ich	766	4.7	0.459	25.2	LOS B	15.6	113.4	0.75	0.66	42.3
North:	Wigram S	treet (north)									
7	L2	65	0.0	0.124	38.7	LOS C	2.8	19.6	0.78	0.72	34.0
8	T1	71	0.0	0.243	39.7	LOS C	4.6	32.8	0.85	0.70	32.1
9	R2	27	3.7	0.243	44.3	LOS D	4.6	32.8	0.85	0.70	33.3
Approa	ich	163	0.6	0.243	40.0	LOS C	4.6	32.8	0.82	0.71	33.0
West: F	Parkes Str	reet (west)									
10	L2	42	0.0	0.464	18.6	LOS B	16.8	120.8	0.58	0.54	45.1
11	T1	704	3.7	0.464	13.3	LOS A	16.8	120.8	0.62	0.58	48.7
12	R2	106	1.9	0.464	19.5	LOS B	8.6	61.5	0.71	0.67	43.8
Approa	ich	852	3.3	0.464	14.3	LOS A	16.8	120.8	0.63	0.59	47.8
All Veh	icles	2020	3.3	0.464	23.5	LOS B	16.8	120.8	0.71	0.64	42.0

Level of Service (LOS) Method: Delay (RTA NSW).

Vehicle movement LOS values are based on average delay per movement

Intersection and Approach LOS values are based on average delay for all vehicle movements.

SIDRA Standard Delay Model is used. Control Delay includes Geometric Delay.

Gap-Acceptance Capacity: SIDRA Standard (Akçelik M3D).

HV (%) values are calculated for All Movement Classes of All Heavy Vehicle Model Designation.

Mover	nent Performance - Pedestrians							
Mov ID	Description	Demand Flow ped/h	Average Delay sec	Level of Service	Average Back Pedestrian ped	of Queue Distance m	Prop. Queued	Effective Stop Rate per ped
P1	South Full Crossing	3	24.7	LOS C	0.0	0.0	0.64	0.64
P2	East Full Crossing	43	38.5	LOS D	0.1	0.1	0.80	0.80
P3	North Full Crossing	16	13.1	LOS B	0.0	0.0	0.47	0.47
P4	West Full Crossing	55	38.5	LOS D	0.2	0.2	0.80	0.80
All Ped	estrians	117	34.7	LOS D			0.75	0.75

Level of Service (LOS) Method: SIDRA Pedestrian LOS Method (Based on Average Delay) Pedestrian movement LOS values are based on average delay per pedestrian movement. Intersection LOS value for Pedestrians is based on average delay for all pedestrian movements.

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### Site: Existing PM

Wigram Street & Parkes Street Intersection

Signals - Fixed Time Isolated Cycle Time = 120 seconds (User-Given Cycle Time)

Movement Performance - Vehicles											
Mov ID	OD Mov	Demand Total veh/h	Flows HV %	Deg. Satn v/c	Average Delay sec	Level of Service	95% Back ( Vehicles veh	of Queue Distance m	Prop. Queued	Effective Stop Rate per veh	Average Speed km/h
South: V	Vigram Stree	t (south)									
1	L2	50	4.0	0.095	33.9	LOS C	2.3	16.4	0.72	0.69	35.7
2	T1	65	0.0	0.357	43.3	LOS D	5.3	37.1	0.88	0.74	30.8
3	R2	45	2.2	0.357	49.6	LOS D	5.3	37.1	0.90	0.74	31.5
Approac	h	160	1.9	0.357	42.1	LOS C	5.3	37.1	0.84	0.73	32.4
East: Pa	rkes Street (	east)									
4	L2	58	0.0	0.390	25.9	LOS B	13.3	94.6	0.67	0.62	41.1
5	T1	678	2.1	0.390	20.3	LOS B	13.4	95.4	0.67	0.60	44.8
Approach		736	1.9	0.390	20.8	LOS B	13.4	95.4	0.67	0.60	44.5
North: W	/igram Street	t (north)									
7	L2	51	0.0	0.114	42.6	LOS D	2.3	16.2	0.81	0.72	32.8
8	T1	109	0.0	0.391	43.8	LOS D	7.3	51.1	0.90	0.75	31.0
9	R2	35	0.0	0.391	48.4	LOS D	7.3	51.1	0.90	0.75	32.1
Approac	h	195	0.0	0.391	44.3	LOS D	7.3	51.1	0.88	0.74	31.6
West: Pa	arkes Street	(west)									
10	L2	58	0.0	0.397	15.5	LOS B	13.2	94.4	0.50	0.48	46.7
11	T1	628	2.5	0.397	10.2	LOS A	13.2	94.4	0.53	0.52	50.6
12	R2	105	1.9	0.397	16.3	LOS B	7.2	51.2	0.62	0.61	45.4
Approac	h	791	2.3	0.397	11.4	LOS A	13.2	94.4	0.54	0.53	49.6
All Vehic	les	1882	1.9	0.397	21.1	LOS B	13.4	95.4	0.65	0.60	43.2

Level of Service (LOS) Method: Delay (RTA NSW).

Vehicle movement LOS values are based on average delay per movement

Intersection and Approach LOS values are based on average delay for all vehicle movements.

SIDRA Standard Delay Model is used. Control Delay includes Geometric Delay.

Gap-Acceptance Capacity: SIDRA Standard (Akçelik M3D).

HV (%) values are calculated for All Movement Classes of All Heavy Vehicle Model Designation.

Movement Performance - Pedestrians												
Mov ID	Description	Demand Flow ped/h	Average Delay sec	Level of Service	Average Back Pedestrian ped	of Queue Distance m	Prop. Queued	Effective Stop Rate per ped				
P1	South Full Crossing	12	21.0	LOS C	0.0	0.0	0.59	0.59				
P2	East Full Crossing	27	42.6	LOS E	0.1	0.1	0.84	0.84				
P3	North Full Crossing	16	10.8	LOS B	0.0	0.0	0.43	0.43				
P4	West Full Crossing	47	42.6	LOS E	0.1	0.1	0.84	0.84				
All Pedestrians		102	35.1	LOS D			0.75	0.75				

Level of Service (LOS) Method: SIDRA Pedestrian LOS Method (Based on Average Delay) Pedestrian movement LOS values are based on average delay per pedestrian movement. Intersection LOS value for Pedestrians is based on average delay for all pedestrian movements.

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# Site: Proposed AM

Wigram Street & Parkes Street Intersection Signals - Fixed Time Isolated Cycle Time = 120 seconds (User-Given Cycle Time)

Movement Performance - Vehicles											
Mov	OD	Demand	Flows	Deg.	Average	ge Level of	95% Back	of Queue	Prop.	Effective	Average
ID	Mov	Total	ΗV	Satn	Delay	Service	Vehicles	Distance	Queued	Stop Rate	Speed
		veh/h	%	v/c	sec		veh	m		per veh	km/h
South	: Wigram S	treet (south)									
1	L2	27	7.4	0.131	35.0	LOS C	3.2	22.8	0.74	0.63	36.2
2	T1	172	0.0	0.490	38.8	LOS C	8.1	56.5	0.85	0.71	32.3
3	R2	40	0.0	0.490	47.0	LOS D	8.1	56.5	0.90	0.75	32.5
Appro	ach	239	0.8	0.490	39.8	LOS C	8.1	56.5	0.84	0.71	32.7
East: I	Parkes Stre	eet (east)									
4	L2	36	5.6	0.487	32.9	LOS C	16.2	118.1	0.78	0.70	38.2
5	T1	730	4.7	0.487	27.3	LOS B	16.3	118.7	0.78	0.69	41.3
Approach		766	4.7	0.487	27.6	LOS B	16.3	118.7	0.78	0.69	41.2
North: Wigram Street (north)											
7	L2	84	0.0	0.147	36.6	LOS C	3.5	24.7	0.76	0.73	34.7
8	T1	71	0.0	0.347	44.3	LOS D	5.9	41.5	0.90	0.75	30.7
9	R2	45	2.2	0.347	48.9	LOS D	5.9	41.5	0.90	0.75	31.8
Appro	ach	200	0.5	0.347	42.1	LOS C	5.9	41.5	0.84	0.74	32.5
West:	Parkes Str	eet (west)									
10	L2	49	0.0	0.489	20.4	LOS B	18.2	130.9	0.62	0.58	44.0
11	T1	704	3.7	0.489	15.1	LOS B	18.2	130.9	0.66	0.61	47.5
12	R2	106	1.9	0.489	21.1	LOS B	9.1	65.7	0.75	0.69	42.9
Appro	ach	859	3.3	0.489	16.1	LOS B	18.2	130.9	0.67	0.62	46.7
All Vel	nicles	2064	3.2	0.490	25.6	LOS B	18.2	130.9	0.75	0.67	40.9

Level of Service (LOS) Method: Delay (RTA NSW).

Vehicle movement LOS values are based on average delay per movement

Intersection and Approach LOS values are based on average delay for all vehicle movements.

SIDRA Standard Delay Model is used. Control Delay includes Geometric Delay.

Gap-Acceptance Capacity: SIDRA Standard (Akçelik M3D).

HV (%) values are calculated for All Movement Classes of All Heavy Vehicle Model Designation.

Movement Performance - Pedestrians											
Mov	Description	Demand	Average	Level of	Average Back	of Queue	Prop.	Effective			
ID	Description	Flow ped/h	Delay sec	Service	Pedestrian ped	Distance m	Queued	Stop Rate per ped			
P1	South Full Crossing	3	26.7	LOS C	0.0	0.0	0.67	0.67			
P2	East Full Crossing	43	36.1	LOS D	0.1	0.1	0.78	0.78			
P3	North Full Crossing	16	14.5	LOS B	0.0	0.0	0.49	0.49			
P4	West Full Crossing	55	36.1	LOS D	0.1	0.1	0.78	0.78			
All Pedestrians		117	32.9	LOS D			0.73	0.73			

Level of Service (LOS) Method: SIDRA Pedestrian LOS Method (Based on Average Delay) Pedestrian movement LOS values are based on average delay per pedestrian movement.

Intersection LOS value for Pedestrians is based on average delay for all pedestrian movements.

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# Site: Proposed PM

Wigram Street & Parkes Street Intersection Signals - Fixed Time Isolated Cycle Time = 120 seconds (User-Given Cycle Time)

Movement Performance - Vehicles											
Mov	OD	Demand	Flows	Deg.	Average	Level of	95% Back	of Queue	Prop.	Effective	Average
ID	Mov	Total	ΗV	Satn	Delay	Service	Vehicles	Distance	Queued	Stop Rate	Speed
		veh/h	%	v/c	sec		veh	m		per veh	km/h
South:	Wigram S	treet (south)									
1	L2	50	4.0	0.098	33.9	LOS C	2.4	16.9	0.72	0.69	35.8
2	T1	65	0.0	0.366	43.7	LOS D	5.2	36.8	0.88	0.74	30.7
3	R2	45	2.2	0.366	50.6	LOS D	5.2	36.8	0.91	0.75	31.3
Approa	ach	160	1.9	0.366	42.6	LOS D	5.2	36.8	0.84	0.73	32.3
East: F	Parkes Stre	eet (east)									
4	L2	58	0.0	0.404	27.3	LOS B	13.8	97.8	0.70	0.64	40.5
5	T1	678	2.1	0.404	21.7	LOS B	13.8	98.6	0.70	0.62	44.0
Approach		736	1.9	0.404	22.2	LOS B	13.8	98.6	0.70	0.62	43.7
North: Wigram Street		treet (north)									
7	L2	58	0.0	0.121	41.0	LOS C	2.6	18.1	0.80	0.72	33.3
8	T1	109	0.0	0.421	45.0	LOS D	7.8	54.4	0.92	0.76	30.6
9	R2	42	0.0	0.421	49.5	LOS D	7.8	54.4	0.92	0.76	31.8
Approa	ach	209	0.0	0.421	44.8	LOS D	7.8	54.4	0.88	0.75	31.6
West:	Parkes Str	eet (west)									
10	L2	76	0.0	0.416	16.6	LOS B	14.3	102.0	0.53	0.51	45.9
11	T1	628	2.5	0.416	11.3	LOS A	14.3	102.0	0.56	0.55	49.8
12	R2	105	1.9	0.416	17.3	LOS B	7.7	55.2	0.64	0.62	44.9
Approa	ach	809	2.2	0.416	12.6	LOS A	14.3	102.0	0.57	0.55	48.7
All Ver	nicles	1914	1.8	0.421	22.3	LOS B	14.3	102.0	0.67	0.62	42.5

Level of Service (LOS) Method: Delay (RTA NSW).

Vehicle movement LOS values are based on average delay per movement

Intersection and Approach LOS values are based on average delay for all vehicle movements.

SIDRA Standard Delay Model is used. Control Delay includes Geometric Delay.

Gap-Acceptance Capacity: SIDRA Standard (Akçelik M3D).

HV (%) values are calculated for All Movement Classes of All Heavy Vehicle Model Designation.

The results of iterative calculations indicate a somewhat unstable solution. See the Diagnostics section in the Detailed Output report.

Movement Performance - Pedestrians												
Mov	Description	Demand	Average	Level of	Average Back	of Queue	Prop.	Effective				
ID	Description	FIOW ned/h	Delay	Service	Pedestrian	Distance	Queued	Stop Rate				
D1	South Full Crossing	10	200	1.08.0		0.0	0.61					
PI	South Full Crossing	12	22.2	LUS C	0.0	0.0	0.01	0.01				
P2	East Full Crossing	27	40.9	LOS E	0.1	0.1	0.83	0.83				
P3	North Full Crossing	16	11.7	LOS B	0.0	0.0	0.44	0.44				
P4	West Full Crossing	47	40.9	LOS E	0.1	0.1	0.83	0.83				
All Pedestrians		102	34.1	LOS D			0.74	0.74				

Level of Service (LOS) Method: SIDRA Pedestrian LOS Method (Based on Average Delay) Pedestrian movement LOS values are based on average delay per pedestrian movement. Intersection LOS value for Pedestrians is based on average delay for all pedestrian movements.

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